

Capital Projects & Purchasing Department 113 Mable T. Willis Blvd. Walterboro, SC 29488 843.539.1968

BID: CTC-13 2018 Dirt Road Paving Durango Lane, Dakota Circle, and Angler Lane

BID DUE: Tuesday, December 11, 2018 @ 11:00am

ADDENDUM 1 DATED: DECEMBER 4, 2018

The following questions have been submitted for clarification:

- 1) Is there a bidder's list available? Answer: NO
- 2) Can the bid date be changed to 12/12 due to the SCDOT letting on 12/11? Answer: NO. Bid date will remain December 11, 2018 at 11am.
- 3) Is a CAD file available prior to bidding? Answer: NO
- 4) There are moving items 1&2 listed on the bid from, but I did not see anywhere that identified what the items were. What are these items? Answer: See Sheet 4, General Notes #18 and #19.
- 5) Will DBE & WBE suppliers count toward the DBE goals? If so, will they count 100%?

 Answer: Yes, these will count as long as they are on the SCDOT approved list of DBE/WBE's.

 All dollars paid directly to the DBE/WBE will be counted towards the goal.
- 6) What pay item will the GABC mucking detail on sheet 4 of both plans be paid under? Maintenance stone? Or will it need to be included in muck excavation? Answer: Include stone noted in the detail in the MUCK EXCAVATION bid line item.
- 7) Please confirm that we are to clear to the right of way line for both projects per the bid forms. In some areas the Ex. Property Line = R/W line is outside of the NPDES line and way outside of the area needed to install the roadway and ditches. Answer: All clearing will be within the NPDES line which generally follows the ROW line. It should also be noted in most locations; the existing ROW is clear and the proposed centerline typically follows the existing and is centered in the ROW.
- 8) Is the borrow material quantity to be used for build-up of the centerline or shoulders? It does not appear as though the quantity is warranted. Answer: Centerline build-up will be necessary in locations as shown on the drawings. The unclassified excavation material may be suitable for shoulders as well as the roadway; however, will be determined during construction. Attached to this addendum is the geotechnical report for your use.

Colleton County Dirt Road Paving Project Colleton County, South Carolina

July 20, 2018 Terracon Project No. 73175080

Prepared for:

Mead & Hunt, Inc. Lexington, South Carolina

Prepared by:

Terracon Consultants, Inc. Columbia, South Carolina

Offices Nationwide Employee-Owned Established in 1965 terracon.com



July 20, 2018



Mead & Hunt, Inc. 878 South Lake Drive Lexington, South Carolina 29072

Attn: Mr. Zack Haney, P.E.

Re: Geotechnical Engineering Report

Colleton County Dirt Road Paving Project

Colleton County, South Carolina Terracon Project No. 73175080

Dear Mr. Haney:

Terracon Consultants, Inc. (Terracon) has completed the geotechnical engineering services for the above referenced project. This study was performed in general accordance with our Proposal No. P73175080, dated May 23, 2017 and authorized on April 11, 2018.

This report presents the findings of the subsurface exploration and provides geotechnical recommendations concerning earthwork and the design and construction of the pavements for the proposed project.

We appreciate the opportunity to be of service to you on this project. If you have any questions concerning this report or we may be of further service, please contact us.

Sincerely,

Terracon Consultants, Inc.

Joseph D.M. Fredendall, E.I.T. Staff Engineer

Phillip A. Morrison, P.E. Geotechnical Department Manager SC Registration No. 17275

Attachments:

Copies: Addressee (1 via email) File (1)



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GEOTECHNICAL ENGINEERING REPORT COLLETON COUNTY DIRT ROAD PAVING PROJECT COLLETON COUNTY, SOUTH CAROLINA

Terracon Project No. 73175080 July 20, 2018

1.0 INTRODUCTION

This report presents the results of our geotechnical engineering services performed for the planned paving of Durango Lane, Dakota Circle, Angler Lane, and Honor Lane in Colleton County, South Carolina. The purpose of these services is to provide information and geotechnical engineering recommendations relative to:

subsurface soil conditions

groundwater conditions

pavements

earthwork

Our geotechnical engineering scope of work for this project included advancing nineteen soil test borings to an approximate depth of 5 feet below the existing roadway surface. Logs of the borings, the Site Location Map and the Boring Location Plan are included in Appendix A of this report. The results of the laboratory testing performed on soil samples obtained from the site during the field exploration are included in Appendix B of this report. Descriptions of the field exploration and laboratory testing are included in their respective appendices.

2.0 PROJECT DESCRIPTION

2.1 Project Description

Item	Description				
Pavements	The project will include the surfacing of Durango Lane, Dakota Circle, Angler Lane, and Honor Lane in Colleton County, South Carolina. Honor Lane, Dakota Circle, and Angler Lane have no outlets, Durango Lane serves as access for Dakota Circle, Angler Lane, and Mackerel Court.				
Grading	Minimal grading in expected to reshape the existing roadway surface for paving, likely less than 1 foot.				
Cut and fill slopes	Assumed to be no steeper than 3H:1V (Horizontal to Vertical)				
Traffic data	We were provided with the following traffic data for the pavement thickness determination: Traffic count: 400 to 600 vehicles per day (use 500 vehicles for analysis) Truck percentage: 6%				



2.2 Site Location and Description

Item	Description					
Location	Durango Lane, Dakota Circle, and Angler Lane are existing rural roads in northeast Colleton County, South Carolina. Honor Lane is an existing rural road in eastern Colleton County. The roadway locations are shown on Exhibits A-1 and A-2 in Appendix A. The approximate road lengths are as follows: Durango Lane - 4,280 LF Dakota Circle - 1,580 LF Angler Lane - 1,215 LF Honor Lane - 3,430 LF					
Existing Improvements	The subject roadways are two-lane, unpaved roadways. Drainage ditches are present along the shoulders of the roadways.					
Current ground cover	Each roadway has a soil surface.					
Existing topography	Detailed topography has not been provided. Based on USGS data and our field observations, the topography along each roadway is relatively flat.					
Existing roadway condition	At the time of our exploration Durango Lane, Dakota Circle, and Angler Lane had just been regraded and appear to generally in good condition. Honor Lane appears to generally be in fair to good condition with some areas of rutting and potholes near the intersection with State Road S-15-91.					
Utilities	Fiber-optic communications lines run within the roadways. An abandoned coaxial cable line was observed in some areas where it had been exposed by previous roadway maintenance. No other utilities were present at the time of our exploration.					

3.0 SUBSURFACE CONDITIONS

3.1 Geology

The site is located in the lower western Coastal Plain physiographic province of South Carolina. The Coastal Plain is a wedge-shaped cross-section of water and wind deposited soil. Its thickness ranges from a featheredge at the surface contact of the Piedmont (Fall Line) to several thousand feet at the present-day coastline. The sediments range in age from the Cretaceous and Tertiary periods at the contact with the bedrock to the recent period at the present coastline. The sediments include clays, silts, sands, and gravels, as well as organics.



3.2 Typical Subsurface Profile

Specific conditions encountered at each boring location are indicated on the individual boring logs included in Appendix A of this report. Stratification boundaries on the boring logs represent the approximate location of changes in soil types; in situ, the transition between materials may be gradual. Based on the results of the borings, subsurface conditions on the project site can be generalized as follows:

3.2.1 Durango Lane, Dakota Circle and Angler Lane (Borings B-1 to B-12)

Description	Approximate Depth to Bottom of Stratum (feet)	Material Encountered	Consistency/Density			
Stratum 1	1-1/2 to 3-1/2	Silty sand	Loose to medium dense			
Stratum 2	5 ¹	Clayey sand	Loose to medium dense			

^{1.} Maximum depth of exploration

3.2.2 Honor Lane (Borings B-13 to B-19)

Stratum 1 5 ¹ Silty sand Loose to medium dense	Description	Approximate Depth to Bottom of Stratum (feet)	Material Encountered	Consistency/Density		
	Stratum 1	5 ¹	Silty sand	Loose to medium dense		

^{1.} Maximum depth of exploration

3.3 Groundwater Conditions

Groundwater was not observed in any test boring at the time of field exploration. These observations represent groundwater conditions at the time of the field exploration and may not be indicative of other times, or at other locations. Groundwater conditions can change with varying seasonal and weather conditions, and other factors.

4.0 RECOMMENDATIONS FOR DESIGN AND CONSTRUCTION

4.1 General Discussion

The boring data indicates existing subgrade conditions that are generally compatible with the intended construction of a flexible pavement. As the roadway subgrades are loose to medium dense, initial compaction of the subgrades should be planned throughout the length of the roadway. Proofrolling after the completion of the compaction process will aid in identifying areas that additional work. The areas containing the loosest soils may require additional reworking such as partial removal and replacement of the upper portion of the layer may be needed to directly expose the lower portion to compactive effort and fully

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compact the layer. The boring data indicates this may be needed in such areas as Borings B-3, B-5 and B15.

In the following sections of this report, we have provided geotechnical engineering recommendations for the pavements. The recommendations contained in this report are based upon the results of data presented herein, engineering analyses, and our current understanding of the proposed project.

4.2 Pavements

4.2.1 Pavement Thickness Design

If the proposed roadway subgrades are prepared as described in this report, a conventional flexible pavement section can be used for the proposed project. In order for pavements to perform satisfactorily, the subgrade soils must have sufficient strength and stability to support construction and design traffic loading.

Based on the laboratory data and considering variations in the soil along the roadway and in moisture content throughout the year, the site soils or new compacted structural fill of similar properties should provide a CBR value of at least 3 (SSV of 1.7) once compacted. This value assumes that the existing subgrade soils have been proofrolled in the presence of a Terracon representative to delineate areas of loose subgrade conditions and that those identified areas are repaired. This also assumes that the upper 12 inches of the pavement subgrade will be compacted to at least 95 percent of the material's standard Proctor maximum dry unit weight. Finally, it presumes that the civil design will provide adequate drainage to avoid the saturation of the pavement subgrades.

The following pavement sections are based on the provided traffic loading, frequencies, and pavement design life provided to us. The information provided to us includes the following:

Design load: 500 vehicles per day

Percentage of trucks: 6%Design period: 20 years

Based on our field observations of the existing site development (predominately rural residential), we have used Road Group C to characterize the truck traffic. Should development conditions of the area served by the subject roads be modified, an alternate road group may apply. The noted traffic loading, volume and design period result in a total pavement loading of about 31,000 ESALs. The recommended pavement section is presented in the table below.

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Pavement Components	Pavement Thickness
HMA Surface Course ¹	200 psy
HMA Intermediate Course ¹	200 psy
Macadam Aggregate Base Course ¹	6 inches
See "General Design Recommendation	ons" section below.

4.2.2 General Design Recommendations

Aggregate base course should be SCDOT Graded Aggregate Base (SCDOT Section 305). Asphaltic cement concrete should be an approved mix design selected from the current SCDOT Standard Type C (SCDOT Sections 402 and 403). Compaction levels of the asphalt and Macadam Base Course materials should conform to SCDOT requirements.

The above sections represent a minimum design thickness and as such periodic maintenance should be anticipated. Higher traffic volumes and/or higher truck percentages may require thicker pavement sections. Pavements should be sloped to provide rapid drainage of surface water. Water allowed to pond on or adjacent to the roadway could saturate the subgrade and contribute to premature pavement deterioration. We recommend preventing construction traffic on areas of partial thickness pavement. Heavy loaded vehicles operation on this surface will cause significant damage resulting in deterioration and reduction in pavement life.

Future performance of pavements constructed on the site will be dependent upon maintaining stable moisture content of the subgrade soils; and, providing for a planned program of preventative maintenance. The performance of all pavements can be enhanced by minimizing excess moisture that can reach the subgrade soils. The following design recommendations should be considered at minimum:

- Site grading at a minimum 2% grade away from the pavements;
- The subgrade and the pavement surface have a minimum ¼ inch per foot cross slope to promote proper surface drainage.

4.2.3 General Construction Considerations

Pavement subgrades carefully evaluated as the time for pavement construction. We recommend the pavement areas be rough graded and then thoroughly proofrolled with a loaded tandem-axle dump truck. Areas where unsuitable conditions are located should be repaired by replacing the materials with properly compacted fill.

Preventative maintenance should be planned and provided through an on-going pavement management program in order to enhance future pavement performance. Preventative maintenance activities are intended to slow the rate of pavement deterioration, and to preserve the pavement investment.

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Preventative maintenance consists of both localized maintenance (e.g. crack and joint sealing and patching) and global maintenance (e.g. surface sealing). Preventative maintenance is usually the first priority when implementing a planned pavement maintenance program and provides the highest return on investment for pavements. Prior to implementing any maintenance, additional engineering observation is recommended to determine the type and extent of preventative maintenance.

4.3 Earthwork

The following presents recommendations for site preparation, excavation, subgrade preparation, and placement of engineered fills on the project. The recommendations presented for design and construction of the pavement system is contingent upon following the recommendations outlined in this section.

Earthwork on this project should be observed and evaluated by Terracon. The evaluation of earthwork should include observation and testing of engineered fill, subgrade preparation, and other geotechnical conditions exposed during the construction of the project.

4.3.1 Site Preparation

Any vegetation and other unsuitable materials should be stripped and removed from the area to be paved. The stripping should extent to at least 5 feet beyond the construction limits. Once the contractor's stripping activities near completion, we recommend that our representative observe the subgrade to identify any remaining pockets of organics that should be removed.

Special precautions should be made to remove all underground utilities and their associated backfill as the new building's foundations or pavements may overlay these materials. Care should be given to locating and addressing these items during the site preparation phase of the project. If overlooked, they could be detrimental to the long-term performance of the building or pavements.

4.3.2 Subgrade Preparation

The exposed subgrades prior to any pavement/base related work should be uniformly compacted to at least 95 percent of the soil's standard Proctor maximum dry unit weight. The compaction of the subgrade soils will require the use of a heavy vibratory pad-foot roller with a minimum energy rating of 60,000 foot-pounds. The number of passes needed to achieve the recommended compaction level will depend on several factors including soil moisture, percentage of fines in the soil, and the applied energy. The as-received samples were moist but may require some moisture conditioning to better facilitate compaction. The target moisture content should be between -2% and +2% of the optimum moisture content. The effectiveness of the compaction efforts should be evaluated in the field. This should include density testing of the in-place compacted soils prior to any fill placement.

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After compaction, the entire subgrade of the roadway should then be proofrolled to check for unstable soil conditions. Proofrolling should be performed with a heavily loaded tandem axle dump truck, scraper or with similar approved construction equipment under the observation of a geotechnical engineer. Any areas that deflect excessively and cannot be stabilized by further rolling should be undercut, the exposed subgrade compacted, and then backfilled with structural fill placed and compacted as recommended herein. Such effort may be necessary in the areas of Borings B-3 B-5 and B-15, as well as other areas of similar soil consistency. Placement of any additional structural fill may commence after the compaction/densification and proofrolling have been successfully completed.

The majority of the roadways are composed of silty sands which can become unstable when exposed to construction traffic, especially after periods of inclement weather or during colder periods of the year. Traffic exposure to wet subgrades can destabilize what would have been otherwise satisfactory conditions, requiring further repair. As a precaution, subgrades should not be exposed to construction traffic after periods of precipitation to allow them to dry. This should help to reduce the amount of subgrade repairs.

4.3.3 Utility Roadway Crossings

Excavations for roadway utility crossings should be based upon the type of pipe and manufacturer's recommendations. Additionally, where necessary, we recommend utility crossings be installed in general accordance with SCDOT Supplemental Technical Specifications for Permanent Pipe Culverts (SC-M-714). Backfill for the excavation should consist of properly compacted fill as defined in Section 4.3.4. Excavations and backfilling for utility crossings should be conducted prior to proofrolling roadway subgrades.

4.3.4 Material Types

Engineered fill should meet the following material property requirements:

Fill Type ¹	USCS Classification	Acceptable Location for Placement				
Imported Structural Fill	SM or SC	All locations and elevations				
On-Site Soils (less organics and debris)	SM or SC	All locations and elevations				

 Controlled, compacted structural fill should consist of approved materials that are free of organic matter and debris. Frozen material should not be used, and fill should not be placed on a frozen subgrade. A sample of each material type should be submitted to the geotechnical engineer for evaluation.

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4.3.5 Compaction Requirements

Item	Description
Fill Lift Thickness	8 inches or less in loose thickness
Compaction Requirements ¹	95 percent of the material's standard Proctor maximum dry density (ASTM D 698)
Moisture Content	Within the range of -2 percent or +2 percent of the optimum moisture content as determined by the standard proctor test at the time of placement and compaction

 We recommend that engineered fill be tested for moisture content and compaction during placement. Should the results of the in-place density tests indicate the specified moisture or compaction limits have not been met, the area represented by the test should be reworked and retested as required until the specified moisture and compaction requirements are achieved.

4.3.6 Excavation

The boring data indicate that the site soils should generally be excavatable to the explored depth of 5 feet using conventional construction equipment. Trenches and other shallow excavations can be performed using rubber-tired back-hoes.

As a minimum, all temporary excavations should be sloped or braced as required by Occupational Health and Safety Administration (OSHA) regulations to provide stability and safe working conditions. Temporary excavations will probably be required during grading operations. The grading contractor, by his contract, is usually responsible for designing and constructing stable, temporary excavations and should shore, slope or bench the sides of the excavations as required, to maintain stability of both the excavation sides and bottom. All excavations should comply with applicable local, state and federal safety regulations, including the current OSHA Excavation and Trench Safety Standards.

Construction site safety is the sole responsibility of the contractor who controls the means, methods and sequencing of construction operations. Under no circumstances shall the information provided herein be interpreted to mean that Terracon is assuming any responsibility for construction site safety or the contractor's activities; such responsibility shall neither be implied nor inferred.

4.3.7 Construction Considerations

Upon completion of filling and grading, care should be taken to maintain the subgrade moisture content prior to construction of pavements. Construction traffic over the completed subgrade should be avoided to the extent practical. The site should also be graded to prevent ponding of surface water on the prepared subgrades or in excavations. If the subgrade should become frozen, desiccated, saturated, or disturbed, the affected material should be removed or these materials should be scarified, moisture conditioned, and recompacted prior to pavement construction.

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The geotechnical engineer should be retained during the construction phase of the project to observe earthwork and to perform necessary tests and observations during subgrade preparation; proofrolling; placement and compaction of controlled compacted fills; backfilling of excavations into the completed subgrade, and just prior to construction of pavements.

5.0 GENERAL COMMENTS

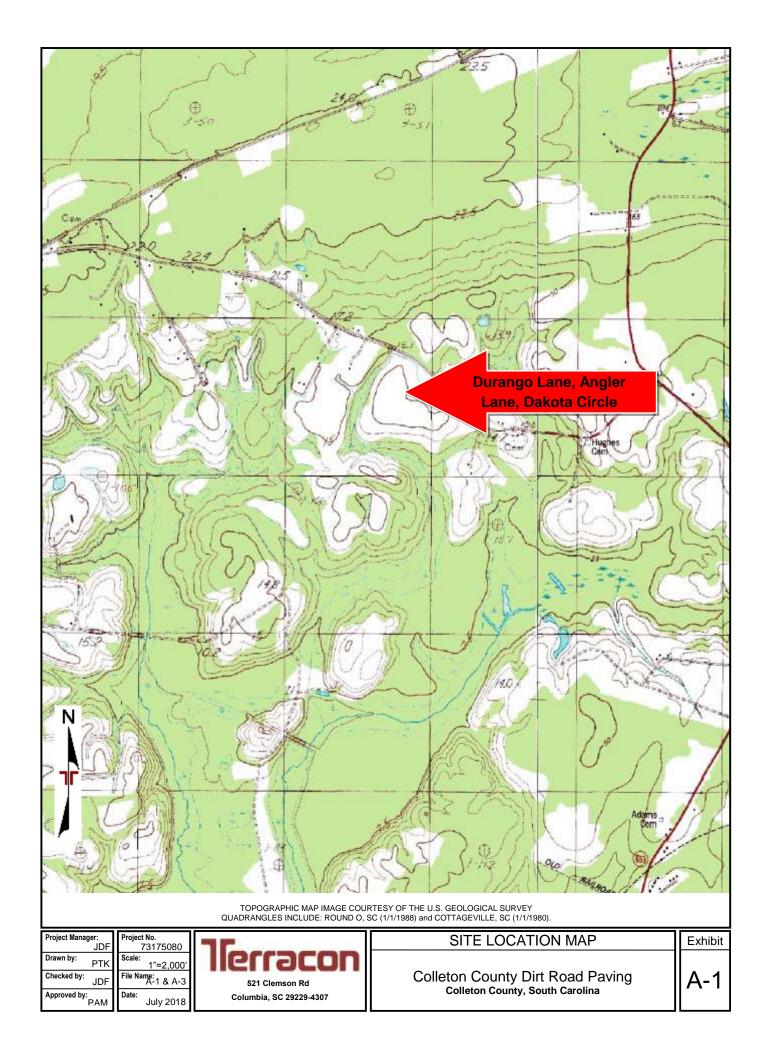
Terracon should be retained to review the final design plans and specifications so comments can be made regarding interpretation and implementation of our geotechnical recommendations in the design and specifications. Terracon also should be retained to provide observation and testing services during grading, excavation, foundation construction and other earth-related construction phases of the project.

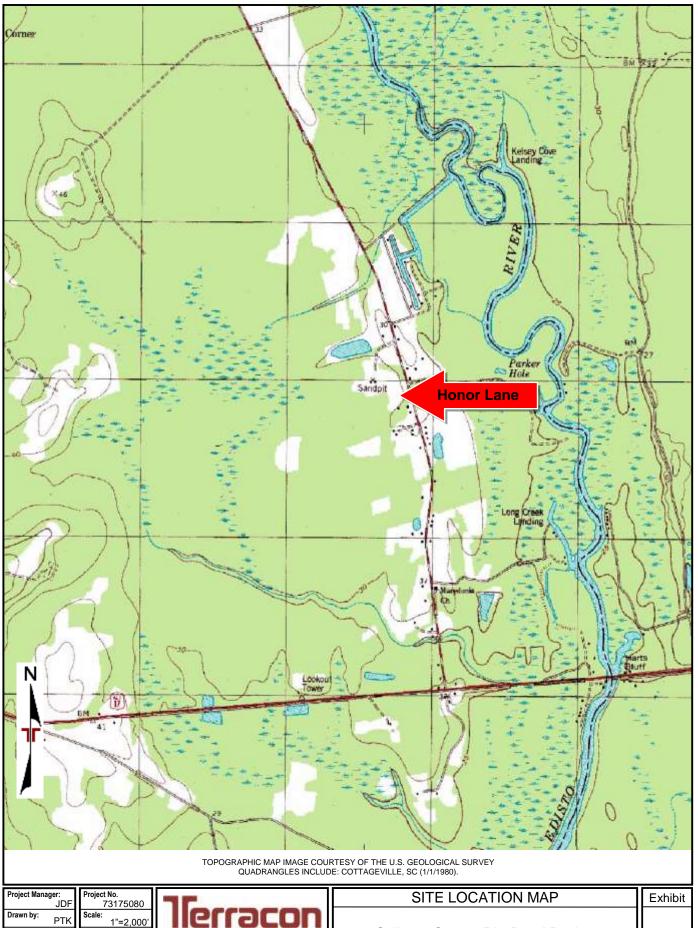
The analysis and recommendations presented in this report are based upon the data obtained from the borings performed at the indicated locations and from other information discussed in this report. This report does not reflect variations that may occur between borings, across the site, or due to the modifying effects of construction or weather. The nature and extent of such variations may not become evident until during or after construction. If variations appear, we should be immediately notified so that further evaluation and supplemental recommendations can be provided.

The scope of services for this project does not include either specifically or by implication any environmental or biological (e.g., mold, fungi, bacteria) assessment of the site or identification or prevention of pollutants, hazardous materials or conditions. If the owner is concerned about the potential for such contamination or pollution, other studies should be undertaken.

This report has been prepared for the exclusive use of our client for specific application to the project discussed and has been prepared in accordance with generally accepted geotechnical engineering practices. No warranties, either express or implied, are intended or made. Site safety, excavation support, and dewatering requirements are the responsibility of others. In the event that changes in the nature, design, or location of the project as outlined in this report are planned, the conclusions and recommendations contained in this report shall not be considered valid unless Terracon reviews the changes and either verifies or modifies the conclusions of this report in writing.

APPENDIX A FIELD EXPLORATION



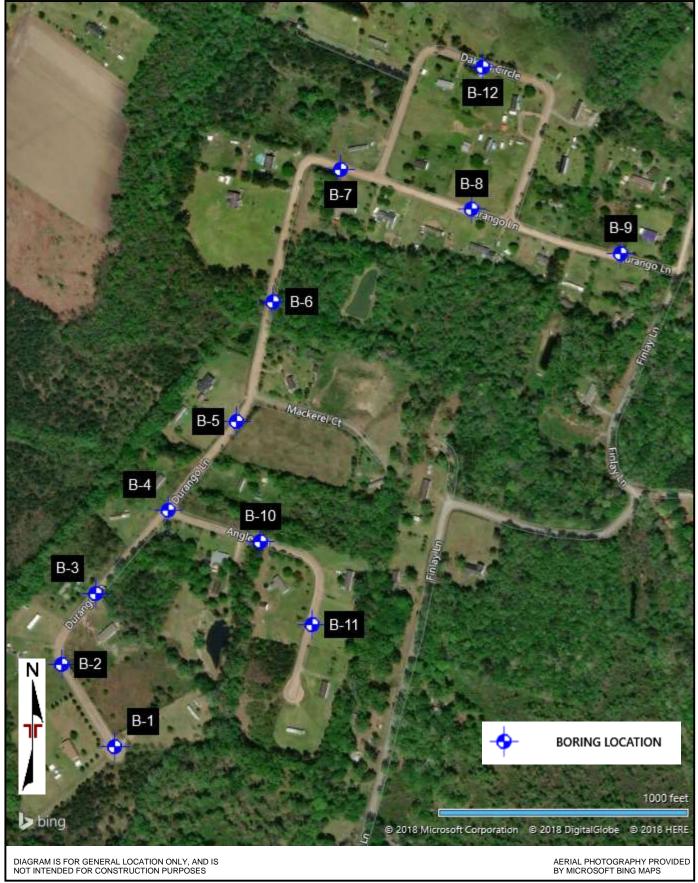


Checked by: File Name: A-2 & A-4 JDF Approved by: PAM July 2018

521 Clemson Rd Columbia, SC 29229-4307

Colleton County Dirt Road Paving Colleton County, South Carolina

A-2



 Project Manager:
 JDF

 JDF
 S

 Drawn by:
 PTK

 Checked by:
 JDF

 Approved by:
 PAM

Project No. 73175080 Scale: AS SHOWN File Name: A-1 & A-3 Date: July 2018

Terracon
521 Clemson Rd

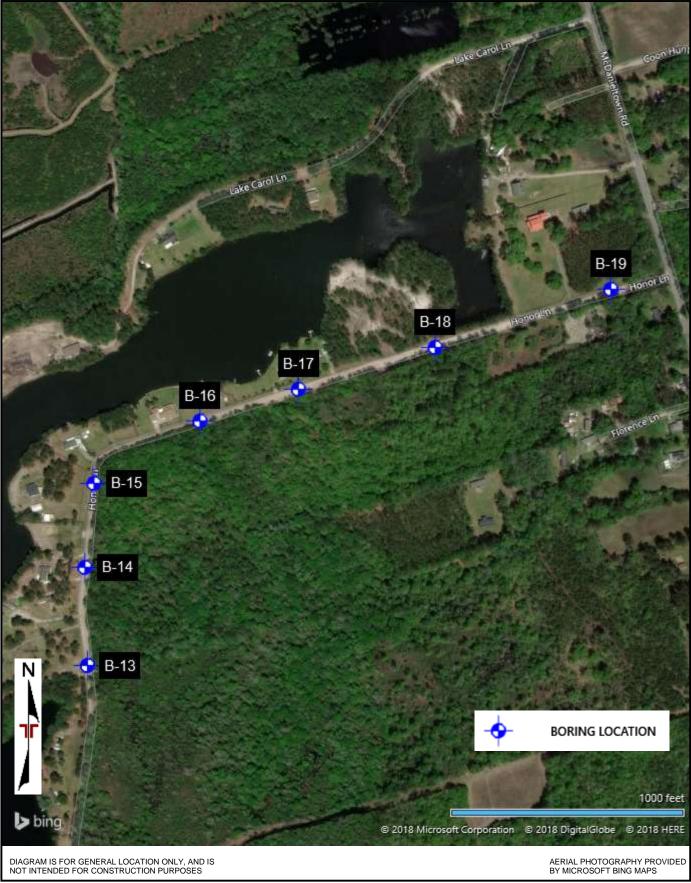
Columbia, SC 29229-4307

EXPLORATION PLAN

Exhibit

Colleton County Dirt Road Paving Colleton County, South Carolina

A-3



Project Manager 73175080 Drawn by: File Name: A-2 & A-4 Checked by: JDF Approved by: PAM July 2018



EXPLORATION PLAN

Colleton County Dirt Road Paving Colleton County, South Carolina

Exhibit

A-4

Colleton County Dirt Road Paving Project Colleton County, SC July 20, 2018 Terracon Project No. 73175080



Field Exploration

Nineteen (19) test borings were drilled at the site on July 18, 2018. The borings were drilled to an approximate depth of 5 feet below the ground surface at the approximate locations shown on the boring location plans, Exhibits A-3 and A-4.

The test borings were located as follows:

Borings	Location	Depths (feet)
B-1 to B-9	Durango Lane	5
B-10 and B-11	Angler Lane	5
B-12	Dakota Circle	5
B-13 to B-19	Honor Lane	5

The borings were located in the field by using the proposed site plan and an aerial photograph of the site. Distances were measured from the various identifiable structures on the noted sources. As no topographic information was provided for the site, no elevations are indicated on the boring logs. The boring locations shown on the Boring Location Plan are approximate and should be considered accurate only to the degree implied by the method of location.

The test borings were advanced with a truck-mounted CME-45C drill rig utilizing 2-¼-inch inside diameter hollow-stem augers. Penetration resistance measurements were obtained by driving the split-spoon samplers into the subsurface materials with a 140-pound automatic hammer falling 30 inches. The penetration resistance value is a useful index in estimating the consistency or relative density of materials encountered. At selected intervals, samples of the subsurface materials were taken by driving split-spoon samplers.

A CME automatic SPT hammer was used to advance the split-barrel sampler in the borings performed on this site. A greater efficiency is typically achieved with the automatic hammer compared to the conventional safety hammer operated with a cathead and rope. Published correlations between the SPT values and soil properties are based on the lower efficiency cathead and rope method. This higher efficiency affects the standard penetration resistance blow count (N) value by increasing the penetration per hammer blow over what would be obtained using the cathead and rope method. The effect of the automatic hammer's efficiency has been considered in the interpretation and analysis of the subsurface information for this report.

Continuous lithologic logs of each boring were recorded by our field engineer during the drilling operations. Representative disturbed soil samples were obtained from the borings and were placed in sealed containers and returned to our laboratory where our engineer visually reviewed and classified them. The purposes of this review were to check the drillers' field classifications and visually estimate the soils' relative constituents (sand, clay, etc.). The soil types and

Colleton County Dirt Road Paving Project • Colleton County, SC July 20, 2018 • Terracon Project No. 73175080



penetrometer values are shown on the Boring Logs. These records represent our interpretation of the field conditions based on the driller's field logs and our engineer's review of the soil samples. The lines designating the interfaces between various strata represent approximate boundaries only, as transitions between materials may be gradual.

Groundwater conditions were evaluated in each boring at the time of site exploration. At the completion of drilling, the boreholes were checked for the presence of groundwater. After which, the borings were backfilled with the auger cuttings.

Our exploration services include storing the collected soil samples and making them available for inspection for 60 days from the report date. The samples will then be discarded unless requested otherwise.

	BORING LOG NO. B-				1 Page 1 of 1							
	PR	OJECT: Colleton County Dirt Road Pav	ing Project	CLIENT: Mead	4 & H	unt	uth	Carolina				
	SIT	E:		Lexii	igion	, 30	utii	Caronna				
		Colleton County, South Caroli	na		ř	1				Lizzana		
	GRAPHIC LOG	LOCATION See Exhibit A-3 Latitude: 32.968° Longitude: -80.514°			DЕРТН (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	LL-PL-PI	PERCENT FINES	
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GPJ IERRACON_DA		2.0			_	_		3-2-5 N=7				
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GEO SMART LOG-NO WELL 73175080 - COLL		·										
D FROM ORIGINAL REPORT.												
AKAIE		Stratification lines are approximate. In-situ, the transition ma	ay be gradual.		Han	nmer T	ype:	Automatic	'	•	•	
JG IS NOT VALID IF SEP	2-1/-	cement Method: 4" Hollow Stem Auger onment Method: ng backfilled with auger cuttings upon completion.	See Exhibit A-5 for des procedures See Appendix B for de procedures and additic See Appendix C for ex abbreviations.		Note	s:						
מם פעו		WATER LEVEL OBSERVATIONS	75		Boring	Starte	d: 06	-18-2018	Boring Com	pleted: 06-18-	-2018	
בעטם		No free water observed at end of drilling		acon	Drill R	ig: CM	E-450	C	Driller: S. B	lair		
2				emson Rd nbia. SC	Proied	t No.: 7	73175	5080	Exhibit:	A-6		

	BORING LOG NO. B-					2 Page 1 of 1							
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	SIT	E:		Lexiii	igtori	, 30	um	Carollila					
		Colleton County, South Caroli	na										
	GRAPHIC LOG	LOCATION See Exhibit A-3 Latitude: 32.9689° Longitude: -80.5147°			DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	ATTERBERG LIMITS LL-PL-PI	PERCENT FINES		
01/	GF	DEPTH			Δ	WA	SAI		8		PER		
ALEMPLATE.GDI //20/		SILTY SAND, fine to medium grained, light be	rown, loose		_			3-4-4 N=8					
J.GPJ IERRACON_UA		3.0			_			3-2-6 N=8					
AD PAVING		CLAYEY SAND , fine to medium grained, light	t reddish brown, loo	ose	=								
A COUNTY DIKE KE					_			3-4-5 N=9					
	. /.	5.0 Boring Terminated at 5 Feet			5 –								
GEO SMART LOG-NO WELL 73175080 - COLL													
D FROM ORIGINAL REPORT.													
ARAIE		Stratification lines are approximate. In-situ, the transition ma	ay be gradual.	-	Han	nmer T	ype:	Automatic	,	•			
JG IS NOT VALID IF SEP	2-1/-	cement Method: " Hollow Stem Auger comment Method: ng backfilled with auger cuttings upon completion.		scription of field escription of laboratory onal data (if any). eplanation of symbols and	Note	s:							
אם דר		WATER LEVEL OBSERVATIONS No free water observed at end of drilling	75		Boring	Starte	d: 06	i-18-2018	Boring Con	pleted: 06-18-	2018		
ווע ב ב		No free water observed at end of drilling		acon	Drill R	ig: CM	E-450	С	Driller: S. E	lair			
2				emson Rd mbia. SC	Proiec	t No.: 7	73175	5080	Exhibit:	A-7			

	BORING LOG NO. B-3					3 Page 1 of 1							
	PR	OJECT: Colleton County Dirt Road Pa	ving Project	CLIENT:	Mead	& Hu	ınt	ıth	Carolina				
	SIT	E:		_	rexiiié	J.O.I.	, 301	ш	Caronna				
		Colleton County, South Caroli	na								Littensen		
m	GRAPHIC LOG	LOCATION See Exhibit A-3 Latitude: 32.9697° Longitude: -80.5142°				DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	LL-PL-PI	PERCENT FINES	
20/15		<u>SILTY SAND</u> , fine to medium grained, light b	prown to light browni	sh red loose			- 0	\					
I A I EMIPLA I E. GU I /		1.5				_			4-3-2 N=5				
G.GPJ LEKKACON_DA		CLAYEY SAND, fine to medium grained, ligh	nt reddish brown, loo	se		_			3-2-3 N=5	22	55-22-33	40	
CAD PAVING						_		\ /					
N COON IY DIKI K		5.0				5 —		\bigvee	3-4-5 N=9				
ARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL 73173080 - COLLE		Stratification lines are approximate. In-situ, the transition m	ay be gradual.			Ham	mer T	ype:	Automatic				
VET.		cement Method:	See Exhibit A-5 for des	scription of field	T	Notes	s:						
2-1/4" Hollow Stem Auger Abandonment Method:			See Appendix B for de- procedures and addition See Appendix C for explaints and additions.	scription of labora anal data (if any).									
פר		WATER LEVEL OBSERVATIONS	75			Boring	Starte	d: 06	-18-2018	Boring Com	pleted: 06-18-	2018	
ב ב ב ב		No free water observed at end of drilling	∣ liGLL	900		Drill Ri	g: CMI	E-450		Driller: S. B	lair		
521 Clemso Columbia					ļ	Project No.: 73175080 Exhibit: A-8							

			BORING L	OG NO. B-	4				ſ	Page 1 of	1
	PR	OJECT: Colleton County Dirt Road Par	ving Project	CLIENT: Mead	d & H	unt	uth	Carolina			
	SIT	 E:		Lexii	igion	, 30	utn	Carollila			
		Colleton County, South Caroli	na		1	1					
	GRAPHIC LOG	LOCATION See Exhibit A-3 Latitude: 32.9706° Longitude: -80.5133°			DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	ATTERBERG LIMITS	PERCENT FINES
20/18	<u> </u>	DEPTH SULTY SAND, fine to medium grained, light b	roun loose			> ö	S				<u> </u>
IAIEMPLAIE.GUI //		SILTY SAND, fine to medium grained, light b			_	_		6-4-5 N=9			
G.GPJ LEKKACON_DA		CLAYEY SAND, fine to medium grained, ligh	t reddish brown, loc	ose	_	-		5-5-3 N=8	15		36
KL ROAD PAVIIN				_		\ /					
UN COON IT DIS		5.0			5-		\bigwedge	4-4-4 N=8			
ORIGINAL REPORT. GEO SMART LOG-NO WELL 731/3080 - COLLI		Boring Terminated at 5 Feet									
AKA I EU FROM ONG		Stratification lines are approximate. In-situ, the transition m	ay be gradual.		Han	nmer T	ype:	Automatic			
JG IS INCT VALID IF SET	2-1/4 Aband	cement Method: 4" Hollow Stem Auger comment Method: ng backfilled with auger cuttings upon completion.	See Exhibit A-5 for des procedures See Appendix B for de procedures and additic See Appendix C for ex abbreviations.		Note	s:					
25		WATER LEVEL OBSERVATIONS No free water absorbed at and of deilling					d: 06	-18-2018	Boring Com	pleted: 06-18-	-2018
ב ב ב ב		No free water observed at end of drilling	acon	Drill R	ig: CM	E-450	0	Driller: S. B	lair		
2			emson Rd nbia. SC	Proiec	t No.: 7	73175	5080	Exhibit:	A-9		

		I	OG NO. E	3-5				ļ	Page 1 of	1	
	PR	OJECT: Colleton County Dirt Road Pav	ring Project	CLIENT: Me	ad & H	unt	uth	Carolina			
	SIT	E:		Le	xirigion	, 30	utii	Carollila			
		Colleton County, South Carolin	na								
	GRAPHIC LOG	LOCATION See Exhibit A-3 Latitude: 32.9716° Longitude: -80.5124°			DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	ATTERBERG LIMITS	PERCENT FINES
0	GR/				出	WAT	SAM	쁜쮼	> 0		PERC
1/20/1		SILTY SAND, fine to medium grained, light br	own, loose				/				
IAIEMPLAIE.GDI					-	_	$\left \right\rangle$	5-3-3 N=6			
G.GPJ LERRACON_DA					-	_		2-3-4 N=7			
KI ROAD PAVIN		3.5 CLAYEY SAND, fine to medium grained, light			\ /						
ON COOM!		5.0						2-3-8 N=11			
GEO SIMARI LOG-NO WELL 73 1/3000 - COLLI	Boring Terminated at 5 Feet										
FROM ORIGINAL REPORT. GEO SIM											
- KA EL		Stratification lines are approximate. In-situ, the transition ma		Han	nmer T	ype:	Automatic				
I VALID IF	2-1/- band	cement Method: 4" Hollow Stem Auger comment Method: ng backfilled with auger cuttings upon completion.	See Exhibit A-5 for des procedures See Appendix B for des procedures and additio See Appendix C for exp abbreviations.	scription of laboratory nal data (if any).		s:					
		WATER LEVEL OBSERVATIONS					d: 06	-18-2018	Boring Com	pleted: 06-18-	-2018
		No free water observed at end of drilling					E-450		Driller: S. B	-	
		521 Clemson Rd Columbia, SC				t No.: 7			Exhibit:		

		1	OG NO. B	-6					Page 1 of	1	
	PR	OJECT: Colleton County Dirt Road Pav	ring Project	CLIENT: Mea	d & Hi	unt	uth	Carolina			
ŀ	SIT			LGAI	ngton	, 30	utii	Caronna			
L		Colleton County, South Carolin	na		1	1				TATTERREDO	
	GRAPHIC LOG	LOCATION See Exhibit A-3 Latitude: 32.9729° Longitude: -80.5119°			DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	LL-PL-PI	PERCENT FINES
81/02/		DEPTH SILTY SAND, fine to medium grained, light br	own loose			-0	\				
DAIAIEMPLAIE.GUI		1.5			_	_		5-5-3 N=8			
IEKKACON		<u>CLAYEY SAND</u> , fine to medium grained, light loose	brownish red, med	lium dense to	_	_		5-7-8 N=15	22	54-21-33	39
KOAD PAVING.GPJ											
COUNTY DIKI		5.0						1-3-6 N=9			
SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL 731/5080 - COLLETON	Advan	Stratification lines are approximate. In-situ, the transition ma				уре:	Automatic				
I VALID IF	2-1/ Aband	cement Method: 4" Hollow Stem Auger onment Method: ng backfilled with auger cuttings upon completion.	See Exhibit A-5 for des procedures See Appendix B for de procedures and addition See Appendix C for exabbreviations.		Note:	s:					
5015		WATER LEVEL OBSERVATIONS				Starte	d: 06	-18-2018	Borina Con	npleted: 06-18-2	-2018
Ž Ž		No free water observed at end of drilling				ig: CM			Driller: S. E	-	
9 SE		521 Clemson Rd Columbia, SC				t No.: 7			Exhibit:		

			BORING L	OG NO. B-	7					Page 1 of	1
	PR	OJECT: Colleton County Dirt Road Par	/ing Project	CLIENT: Mead	l & H	unt	ıth	Carolina			
	SIT	E:		Lexii	igtori	, 301	utii	Caronna			
		Colleton County, South Caroli	na		Г				Ī	LATTERDEDO.	
	GRAPHIC LOG	LOCATION See Exhibit A-3 Latitude: 32.9743° Longitude: -80.511°			DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	LL-PL-PI	PERCENT FINES
20/18		DEPTH SILTY SAND, fine to medium grained, light b	rown looso			>ō	S				<u> </u>
I A I EMIPLA I E. GD I 7		1.5			_		\bigvee	3-4-4 N=8			
GENT LERRACON_UT		CLAYEY SAND , fine to medium grained, ligh	t reddish brown, loo	ose	_	-	\bigvee	6-4-5 N=9			
בואבו הכאבו הבאיוואר				_			225				
		5.0		5-		\bigwedge	3-3-5 N=8				
GEO SMART LOG-NO WELL 731/3080 - COLLE		Boring Terminated at 5 Feet									
ID FROM ORIGINAL REPORT. GE											
AKAIL		Stratification lines are approximate. In-situ, the transition management		Ham	mer T	ype:	Automatic		_		
JG IS NOT VALID IT SET	2-1/4 Aband	cement Method: 4" Hollow Stem Auger comment Method: ng backfilled with auger cuttings upon completion.	See Exhibit A-5 for des procedures See Appendix B for de procedures and additic See Appendix C for exabbreviations.		Note:	S:					
25		WATER LEVEL OBSERVATIONS No free water cheened at end of drilling					d: 06	-18-2018	Boring Com	npleted: 06-18-	2018
בבכם		No free water observed at end of drilling	acon	Drill R	ig: CMI	E-450		Driller: S. B	lair		
2			emson Rd nbia. SC	Proiec	t No.: 7	3175	5080	Exhibit:	A-12		

			BORING L	OG NO	. B-8	3				ı	Page 1 of	1
ſ	PR	OJECT: Colleton County Dirt Road Pav	ing Project	CLIENT:				ıth	Carolina			
ŀ	SIT				Lexiii	gton,	, 300	<i>.</i>	Caronna			
ļ		Colleton County, South Caroli	na				1				ATTERBERG	1
	GRAPHIC LOG	LOCATION See Exhibit A-3 Latitude: 32.9739° Longitude: -80.5093°				DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	LL-PL-PI	PERCENT FINES
20/18		<u>DEPTH</u> <u>SILTY SAND</u> , fine to medium grained, light be	rown. medium dens	<u>е</u>			-0	() \			<u> </u>	<u> </u>
DAIAIEMPLAIE.GDI /		1.5				_		$\left\langle \right\rangle$	6-7-4 N=11			
I EKKACON_		<u>CLAYEY SAND</u> , fine to medium grained, ligh loose	t brownish red, med	lium dense to		_		\bigvee	2-7-4 N=11	17	38-17-21	39
ROAD PAVING.GPJ												
COUNTY DIKE									2-3-6 N=9			
LLEI OF	<u> </u>	Boring Terminated at 5 Feet				5 —		′ (
05 - 08 - 08												
/31/50												
NO WELL 73175080 - COLLETON												
MARIL												
GEO SI												
AL REP												
NE PER PER PER PER PER PER PER PER PER PE												
N N N												
KATED FROM ORIGINAL REPORT. GEO SMART LOG-		Stratification lines are approximate. In-situ, the transition ma			Ham	mer Ty	/pe:	Automatic			<u></u>	
		cement Method:	scription of field		Notes	S:						
VALID IF	Z-1/4	4" Hollow Stem Auger	procedures See Appendix B for desprocedures and addition	scription of labora	atory							
_ ⊦		andonment Method: See Appendix C for explanation abbreviations.										
		WATER LEVEL OBSERVATIONS No free vector observed at earl of drilling					Started	d: 06	-18-2018	Boring Com	pleted: 06-18-	2018
BCKI		ivo iree water observed at end of drilling	No free water observed at end of drilling				g: CME	E-450	C	Driller: S. B	lair	
2		521 Clemson Rd Columbia. SC				Proiect	No.: 7	3175	5080	Exhibit:	————— A-13	

			BORING L). B-9	•					Page 1 of	1	
	PR	OJECT: Colleton County Dirt Road Pa	ving Project	CLIENT:	Mead	& Hu	ınt	ıth	Carolina			
	SIT	E:		+	rexiii	gton	, 300	atri	Caronna			
		Colleton County, South Caroli	na		ı					ī	LATTERDEDO.	
	GRAPHIC LOG	LOCATION See Exhibit A-3 Latitude: 32.9734° Longitude: -80.5074°				DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	LL-PL-PI	PERCENT FINES
20/18		DEPTH SILTY SAND, fine to medium grained, light b	rown loose				>0	S				<u> </u>
IAIEMPLAIE.GUI //		1.5				_		$\sqrt{}$	6-5-4 N=9			
J.GPJ IERRACON_UA		CLAYEY SAND, fine to medium grained, ligh	it reddish brown, me	edium dense		_		\bigvee	2-4-8 N=12			
IKI KOAD PAVIING				_								
UN COUNTT D				5 —		\bigwedge	3-3-7 N=10					
GEO SMARTI LOG-NO WELL 73175080 - COLLE		Boring Terminated at 5 Feet										
D FROM ORIGINAL REPORT. GEO S												
AKAIE		Stratification lines are approximate. In-situ, the transition m		1	Ham	mer Ty	ype:	Automatic	I	1	-	
JG IS NOT VALID IF SEF	2-1/4 Aband	cement Method: 4" Hollow Stem Auger comment Method: ng backfilled with auger cuttings upon completion.	See Exhibit A-5 for des procedures See Appendix B for de procedures and additio See Appendix C for ex abbreviations.	escription of labor onal data (if any).		Notes	S :					
פור		WATER LEVEL OBSERVATIONS No free water observed at end of drilling					Starte	d: 06	-18-2018	Boring Com	pleted: 06-18-	-2018
ב ב ב ב ב		No free water observed at end of drilling	300		Drill Ri	g: CME	E-450	C	Driller: S. B	lair		
2				Proiect	t No.: 7	'3175	5080	Exhibit:	A-14			

		i	BORING LO	OG NO. B-1	10					Page 1 of	1
	PR	OJECT: Colleton County Dirt Road Pa	ving Project	CLIENT: Mead	l & Hu	unt	uth	Carolina			
	SIT			Lexii	igton	, 50	utii	Oaronna			
		Colleton County, South Caroli	na						1	LATTERDEDO.	
	GRAPHIC LOG	LOCATION See Exhibit A-3 Latitude: 32.9703° Longitude: -80.5121°			DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	LL-PL-PI	PERCENT FINES
20/18		DEPTH SILTY SAND, fine to medium grained, light b	rown looso			>ō	S				<u> </u>
IAIEMPLAIE.GUI //		1.5			_	_		3-5-3 N=8			
SEPU LERRANCON_LOA		<u>CLAYEY SAND</u> , fine to medium grained, ligh dense	it reddish brown, loo	se to medium	_	-		3-4-3 N=7			
אוויארי טאטא וא				_	-						
ON COON I I		5.0						5-4-7 N=11			
 GEO SIMARTI LOG-INO WELL 73175080 - COLL 	Boring Terminated at 5 Feet										
I LI L'ACIM ONIGINAL REPORT		Chatification line are according to the its Ata Assocition		l law			Automotiv				
ARA		Stratification lines are approximate. In-situ, the transition m		Ham	ımer f	ype:	Automatic				
JG IS NOT VALID IF SEF	2-1/4 Aband	cement Method: 4" Hollow Stem Auger conment Method: ng backfilled with auger cuttings upon completion.	scription of field scription of laboratory anal data (if any). planation of symbols and	Notes	S:						
פר		WATER LEVEL OBSERVATIONS No free water observed at end of drilling					d: 06	-18-2018	Boring Con	pleted: 06-18-	-2018
ב ב ב ב		No free water observed at end of drilling	acon	Drill Ri	ig: CM	E-450		Driller: S. E	lair		
2			emson Rd nbia. SC	Proiec	t No.: 7	73175	5080	Exhibit:	A-15		

		i	BORING L	OG NO. B-1	11					Page 1 of	1
	PR	OJECT: Colleton County Dirt Road Pa	ving Project	CLIENT: Mead	l & Hi	unt	uth	Carolina			
	SIT	E:		Lexii	igton	, 50	uuii	Caronna			
		Colleton County, South Caroli	na		Г					LATTERDED C	N.
	GRAPHIC LOG	LOCATION See Exhibit A-3 Latitude: 32.9694° Longitude: -80.5114°			DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	ATTERBERG LIMITS	PERCENT FINES
20/18		DEPTH SILTY SAND, fine to medium grained, light b	urown looso			>0	S				<u> </u>
IAIEMPLAIE.GDI //		1.5			_	-		5-3-6 N=9			
SEPU LERRANCON_LOA		<u>CLAYEY SAND</u> , fine to medium grained, ligh dense	it brownish red, loos	e to medium	_	-		3-4-4 N=8			
AL ROAD PAVIING				_	-						
		5.0 Boring Terminated at 5 Feet			5-		\bigvee	3-4-6 N=10			
ORIGINAL REPORT. GEO SMART LOG-NO WELL 73175080 - COLL											
ARA I EU L'ROM ORIGIE		Stratification lines are approximate. In-situ, the transition m	ay be gradual.		Ham	nmer T	ype:	Automatic			
G IS NOT VALID IT SEL	2-1/4 Aband	cement Method: 4" Hollow Stem Auger comment Method: ng backfilled with auger cuttings upon completion.	See Exhibit A-5 for des procedures See Appendix B for de procedures and additic See Appendix C for ex abbreviations.		Note	s:					
20 50		WATER LEVEL OBSERVATIONS No free water changed at end of drilling					d: 06	i-18-2018	Boring Cor	npleted: 06-18-	-2018
בבלם		No free water observed at end of drilling	acon	Drill R	ig: CM	E-450	C	Driller: S. E	Blair		
2			emson Rd nbia. SC	Proiec	t No.: 7	7317	5080	Exhibit:	A-16		

				BORING L	OG NO. B-1	12				í	Page 1 of	1
	PR	OJECT:	Colleton County Dirt Road	Paving Project	CLIENT: Mead	l & H	unt	uth	Carolina		J	
	SIT	E:			Lexii	igion	, 30	utii	Carollila			
			Colleton County, South Car	olina			1				ATTERREDO	1
	2		N See Exhibit A-3 .9755° Longitude: -80.5092°			DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	ATTERBERG LIMITS	PERCENT FINES
20/18		DEPTH SILT	Y SAND, fine to medium grained, lig	ht hrown medium dens	•		> 5	S				8
AIEMPLAIE.GUI		1.5	<u>r GANO</u> , line to medium grained, lig	nt brown, mediam dens		_	-		2-4-7 N=11			
G.GPJ IERRACON_DA		<u>CLA</u> ` dens	(EY SAND , fine to medium grained, e	light brownish red, loos	e to medium	_	-		3-4-4 N=8			
IN I Y DIKI KOAD PAVIN					_			1-2-9 N=11				
N COL		5.0				_		$/\!\!/ \mathbb{V}$				
ED FRUM ORIGINAL REPORT. GEO SMART LOG-NO WELL 731/3080 - COLLETO		Borii		5 —								
PARA		Stratification lines are approximate. In-situ, the transition may be gradual.						ype: .	Automatic			
G IS NOT VALID IF SE	2-1/4 Abando	vancement Method: 2-1/4" Hollow Stem Auger See Exhibit A-5 for description of field procedures See Appendix B for description of laboratory procedures and additional data (if any). See Appendix C for explanation of symbols a abbreviations.										
NG LC			R LEVEL OBSERVATIONS		Boring	Starte	d: 06-	-18-2018	Boring Com	pleted: 06-18-	2018	
ב ב ב ב ב		ivo tree v	vater observed at end of drilling	acon	Drill R	ig: CM	E-450	;	Driller: S. B	lair		
2			521 Clemson Rd						080	Evhihit:	Δ_17	

				BORING L	OG NO. B-1	13				i	Page 1 of	1
ı	PR	OJECT:	Colleton County Dirt Road I	Paving Project	CLIENT: Mead	I & H	unt	uth	Carolina		<u> </u>	
	SIT	E:			Lexii	igion	, 00	utii	Oaronna			
		LOCATIO	Colleton County, South Car	olina			. (0	l1			ATTERBERG	ω
2	GRAPHIC LOG		.9383° Longitude: -80.4213°			DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	LIMITS	PERCENT FINES
7/20/18		DEPTH SILT dens	Y SAND, fine to medium grained, ligh	nt brown to light browni	sh red, medium		. 0	\ /				
A I EMIPLA I E. GD I		delle				_	-		6-7-4 N=11			
G.GPJ LERRACON_DAL						_	_		5-7-7 N=14			
NIY DIKI KOAD PAVIN					_			8-8-12 N=20				
		5.0				5 –		$/ \setminus$				
ART LUG-NO WELL 73175080 - COLLE		Волі	ng Terminated at 5 Feet									
FROM ORIGINAL REPORT. GEO SMI												
AKAIED		Stratificati		Han	nmer T	ype:	Automatic					
G IS NOT VALID IF SEP	2-1/-	cement Metr 4" Hollow St onment Metr ng backfilled	em Auger	escription of field escription of laboratory onal data (if any). planation of symbols and	Note	s:						
NG LOC			R LEVEL OBSERVATIONS	75		Boring	Starte	ed: 06-	-18-2018	Boring Com	pleted: 06-18-	2018
בוא בוא		No free v	vater observed at end of drilling	acon	Drill R				Driller: S. B	lair		
2			521 Clemson Rd						5080	Evhihit:	Δ_18	

				BORING L	OG NO. B-1	4				1	Page 1 of	1
	PR	OJECT:	Colleton County Dirt Road F	Paving Project	CLIENT: Mead	l & Hi	unt So	uth	Carolina			
	SIT	E:				gion	, 00		oui oiiiiu			
	(ŋ	LOCATIO	Colleton County, South Card	olina			ıΩ	ш			ATTERBERG LIMITS	S
8	GRAPHIC LOG	Latitude: 32	.9395° Longitude: -80.4213°			DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	LL-PL-PI	PERCENT FINES
1/07//		DEPTH SILT	Y SAND, fine to medium grained, ligh	t brown to light browni	sh red, loose to			/				
A I EMPLA I E. GD I		medi	um dense			_	_		5-9-6 N=15			
G.GPJ LERRACON_DAI						_			4-6-3 N=9			
UNIY DIRI ROAD PAVIN					_			4-4-6 N=10				
ON CO		5.0				5 –		$/\setminus$				
J FRUM URIGINAL REPURT. GEO SMART LOG-NO WELL 731/3080 - COLLE		Вопі	ng Terminated at 5 Feet									
ARAIEL		Stratification lines are approximate. In-situ, the transition may be gradual.						ype:	Automatic			
G IS NOT VALID IF SEP	2-1/-	cement Metr 4" Hollow St onment Metr ng backfilled	em Auger	scription of field scription of laboratory onal data (if any). planation of symbols and	Note	s:						
NG LOC			R LEVEL OBSERVATIONS		Boring	Starte	ed: 06	-18-2018	Boring Com	npleted: 06-18-	-2018	
ב ב ב ב ב		No tree V	water observed at end of drilling	acon	Drill R	ig: CM	E-450	3	Driller: S. B	lair		
2				emson Rd	Projec	t No ·	73175	5080	Evhihit:	Δ_10		

				BORING L	OG NO. B-1	15				ı	Page 1 of	1
	PR	OJECT:	Colleton County Dirt Road F	Paving Project	CLIENT: Mead	I & H	unt So	uth	Carolina			
	SIT	E:				.g.o.	, 00		oui oiiiiu			
ı	(D	LOCATIO	Colleton County, South Card	olina			ω.				ATTERBERG	S
	GRAPHIC LOG		.9404° Longitude: -80.4212°			DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	LIMITS	PERCENT FINES
720/18		DEPTH SILT	Y SAND, fine to medium grained, ligh	t brown to light browni	sh red loose		>ō	S				3
ALEMPLAIE.GUI /				g		_			3-2-3 N=5			
S.GPJ LERKACON_DA						_	_		3-4-3 N=7			
DIKI KOAD PAVIIN					_			2-4-4				
ON COON I		5.0	ng Terminated at 5 Feet			5 –		\bigwedge	N=8			
J WELL 131/3000 - COLLE		Bom	ig reminated at 3 reet									
I. GEO SMARI LOG-N												
FROM ORIGINAL REPOR												
ARAIEL		Stratificati	on lines are approximate. In-situ, the transition		Han	nmer T	ype:	Automatic		1	<u> </u>	
S IS NOT VALID IF SEP	2-1/4 Aband	cement Meth 4" Hollow Sto onment Meth ng backfilled	scription of field escription of laboratory onal data (if any). planation of symbols and	Note	s:							
NG LOC			R LEVEL OBSERVATIONS	77		Boring	Starte	ed: 06	-18-2018	Boring Com	pleted: 06-18-	2018
ש מ מ		No free v	vater observed at end of drilling	acon	Drill R	ig: CM	E-450	C	Driller: S. B	lair		
2			521 Clemson Rd						5080	Evhihit:	Δ-20	

			BORING LO	OG NO. B-1	6				ſ	Page 1 of	1
Р	ROJECT:	Colleton County Dirt Road F	Paving Project	CLIENT: Mead			uth	Carolina			
S	ITE:	Colleton County, South Care	olina		9	,					
GRAPHIC LOG	Latitude: 32	N See Exhibit A-4 .9412° Longitude: -80.4197°			DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	ATTERBERG LIMITS LL-PL-PI	PERCENT FINES
	DEPTH WEL	L GRADED SAND WITH SILT , fine to	medium grained, light	brown, loose	_			2-9-1 N=10			
	2.0 SILT medi	<u>Y SAND,</u> fine to medium grained, ligh um dense	t brown to light brownis	sh red, loose to	-			3-5-4 N=9	15		10
	5.0				_			4-4-7 N=11			
			See Exhibit A-5 for desprocedures See Appendix B for desprocedures and addition	scription of laboratory	Ham Notes		ype:	Automatic			
Aba		nod: with auger cuttings upon completion.	See Appendix C for expabbreviations.	planation of symbols and	<u> </u>	.	1.50	40.0045	D : -		
?		vater observed at end of drilling		acon	<u> </u>			-18-2018	_	pleted: 06-18-	2018
			521 Cle	mson Rd Ibia, SC	Drill Ri				Driller: S. B Exhibit:		

				BORING L	OG NO. B-1	7					Page 1 of	1
Ī	PR	OJECT:	Colleton County Dirt Road Pa	aving Project	CLIENT: Mead	l & Hi	unt	uth	Carolina			
ŀ	SIT	Ē:			Lexiii	igton	, 50	utii	Caronna			
ŀ		ı	Colleton County, South Caro	lina			1				ATTERBERG	
	L0G	LOCATIO	N See Exhibit A-4			<u>:</u>	WATER LEVEL OBSERVATIONS	YPE	ST	(%)	LIMITS	PERCENT FINES
	GRAPHIC LOG	Latitude: 32	.9415° Longitude: -80.4183°			DEPTH (Ft.)	RLE	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)		N F
	зКАР					DE P.	/ATE	AMPI	FIELI	M TNO	LL-PL-PI	RCE
81/n:	. 11.11.	DEPTH					>8	'S		0		8
7//		SILT dens	<u>Y SAND</u> , fine to medium grained, light e	brown to light browni	sh red, medium			$\ \ \ /$				
E.G.								V	3-4-5			
PLA .						_		$ \Lambda $	N=9			
A I EIV								/ \				
L'AN												
						_		$ \setminus / $				
7 2 2 3								X	5-8-11 N=19			
آ - ا								$ / \setminus $				
S G.						_		$\backslash \!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!\!$				
PAV												
KOAL								$\backslash /$				
באום						_	-	$ \setminus $	7-9-11			
<u> </u>								$ \lambda $	N=20			
								$ / \setminus$				
	<u>. . . </u>	5.0 Bori i	ng Terminated at 5 Feet			5 –						
- 080												
31/20												
, 												
Š												
-N-0-1												
SIMARI												
S S S												
<u>ج</u> ا												
Х П												
INAL												
25												
22												
		Stratificati	on lines are approximate. In-situ, the transition i	may be gradual		Ham	nmer T	vne.	Automatic			
A K		Cuamodu	on most are approximate. In out, the transition i	, so gradali.		riuli		,,,,,	. atomatio			
T VE		cement Meth 4" Hollow St		See Exhibit A-5 for des	scription of field	Note	s:					
ALID	/	zz ot	y -	procedures See Appendix B for de	scription of laboratory							
<u>}</u>		onment Meth			planation of symbols and							
200			with auger cuttings upon completion.	abbreviations.								
		WATE	R LEVEL OBSERVATIONS	75		Boring	Starte	d: 06	-18-2018	Borina Com	pleted: 06-18-	2018
בוצן בוצן		No free v	vater observed at end of drilling	⊥ llerr	acon	Drill R				Driller: S. B		_0.0
Ď Λ Ε				521 Cl	emson Rd	Projec				Evhihit:		

				BORING L	OG NO. B-1	8				ı	Page 1 of	1
	PR	OJECT:	Colleton County Dirt Road I	Paving Project	CLIENT: Mead	l & Hi	unt . So	uth	Carolina		_	
	SIT	E:	Colleton County South Con	alina		.g	,					
2	GRAPHIC LOG		Colleton County, South Car N See Exhibit A-4 .942° Longitude: -80.4164°	Ullila		DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	ATTERBERG LIMITS	PERCENT FINES
4 I EMPLA I E. GD I 7/20/18		DEPTH SILT medi	<u>Y SAND</u> , fine to medium grained, ligh um dense	nt brown to light brown	ish red, loose to			8	5-6-11 N=17			Δ.
G.GPJ IERRACON_DAIN						_			9-10-8 N=18	10		16
COUNTY DIKT ROAD PAVIN						_	-		5-8-11 N=19			
D FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL 731/3080 - COLLETOT		5.0 Borii	ng Terminated at 5 Feet			5 —						
ARAIEL		Stratificati	on lines are approximate. In-situ, the transitio	n may be gradual.		Ham	nmer T	ype:	Automatic	<u> </u>	1	
G IS NOT VALID IF SER	2-1/-	cement Metr 4" Hollow St onment Metr ng backfilled	em Auger	See Exhibit A-5 for de procedures See Appendix B for de procedures and addition See Appendix C for exabbreviations.	escription of laboratory	Note	s:					
NG LO			R LEVEL OBSERVATIONS water observed at end of drilling	75		Boring	Starte	ed: 06	-18-2018	Boring Com	pleted: 06-18-	2018
אטם ע		7.07100	.a.c. Jacon rod at ond or drining		SCON	Drill R	ig: CM	E-450	0	Driller: S. B	lair	
ć l					mhia SC	Projec	t No · ·	73175	กลก	Eyhihit:	Δ-23	

				BORING L	OG NO. B-1	9				i	Page 1 of	1
ĺ	PR	OJECT:	Colleton County Dirt Road F	Paving Project	CLIENT: Mead			uth	Carolina		<u> </u>	
ł	SIT	E:			Lexiii	igtori	, 50	utii	Caronna			
ļ			Colleton County, South Card	olina			l				ATTERBERG	
	GRAPHIC LOG		N See Exhibit A-4 .9427° Longitude: -80.414°			DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	FIELD TEST RESULTS	WATER CONTENT (%)	LL-PL-PI	PERCENT FINES
81/07	U U	DEPTH					>8	s/s		O		PE
IAIEMPLAIE.GDI //a		<u>SILT</u> dens	<u>Y SAND</u> , fine to medium grained, ligh e	t brown to light browni	sh red, medium	_	_		3-5-7 N=12			
G.GPJ IERRACON_DA						_	-		6-7-9 N=16			
A COUNTY DIRT ROAD PAVIL		5.0				_	_		3-6-7 N=13			
EPARALED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL 731/3080 - COLLETO		Borii Stratificati	on lines are approximate. In-situ, the transition					уре:	Automatic			
VALID IF OF		cement Meth 4" Hollow St		See Exhibit A-5 for desprocedures See Appendix B for de procedures and addition	scription of laboratory	Note	s:					
JON SI SI		onment Mething backfilled	nod: with auger cuttings upon completion.		planation of symbols and							
NG LC			R LEVEL OBSERVATIONS	75		Boring	Starte	d: 06	-18-2018	Boring Com	pleted: 06-18-	2018
באטם		No tree v	vater observed at end of drilling	– liett	acon	Drill R	ig: CM	E-450		Driller: S. B	lair	
2					emson Rd	Projec	t No · ·	73175	5080	Evhihit:	Δ_24	

APPENDIX B LABORATORY TESTING

Colleton County Dirt Road Paving Project Colleton County, SC July 20, 2018 Terracon Project No. 73175080



Laboratory Testing Description

Samples retrieved during the field exploration were taken to the laboratory for further observation by the project geotechnical engineer and were classified in accordance with the Unified Soil Classification System (USCS) described in Appendix C. At that time, the field descriptions were confirmed or modified as necessary and an applicable laboratory testing program was formulated to determine engineering properties of the subsurface materials.

Laboratory tests were conducted on selected soil samples and the test results are presented in this appendix. Selected bulk samples of the site soils were combined to make composite samples, and these composite samples were tested in the laboratory. The laboratory test results were used for the geotechnical engineering analyses, and the development of foundation and earthwork recommendations. Laboratory tests were performed in general accordance with the applicable ASTM, local or other accepted standards.

Selected soil samples obtained from the site were tested for the following engineering properties:

Percent FinesAtterberg LimitsASTM D1140-06ASTM D4318-10

Moisture Content Determination
 ASTM D2216-10

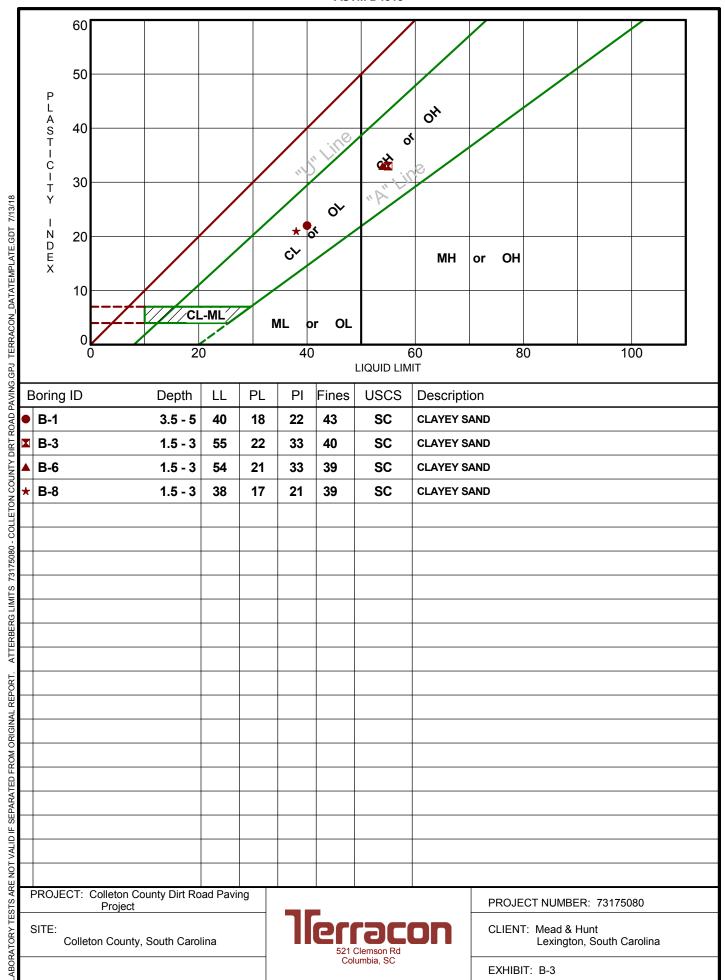
Summary of Laboratory Results

						Sheet 1 of		
BORING ID	Depth (Ft.)	Liquid Limit	Plastic Limit	Plasticity Index	% Fines	Water Content (%)		
B-1	3.5 - 5	40	18	22	43.0	18		
B-3	1.5 - 3	55	22	33	39.9	22		
B-4	1.5 - 3				36.4	15		
B-6	1.5 - 3	54	21	33	39.0	22		
B-8	1.5 - 3	38	17	21	38.8	17		
B-16	1.5 - 3				10.0	15		
B-18	1.5 - 3				15.5	10		
PROJECT: (Colleton County Dir Project	t Road Paving			PROJECT NUMBER: 73	175080		
SITE: Colleton County, South Carolina			Terral		CLIENT: Mead & Hunt Lexington, South Carolina			
			521 Clemson Rd Columbia, SC		EXHIBIT: B-2			



ATTERBERG LIMITS RESULTS

ASTM D4318



APPENDIX C SUPPORTING DOCUMENTS

GENERAL NOTES

DESCRIPTION OF SYMBOLS AND ABBREVIATIONS

		\square		Water Initially Encountered		(HP)	Hand Penetrometer
	Auger	Split Spoon		Water Level After a Specified Period of Time		(T)	Torvane
NG	Challey Tube	Maara Cara	LEVEL	Water Level After a Specified Period of Time	ESTS	(b/f)	Standard Penetration Test (blows per foot)
IPLIN	Shelby Tube	Macro Core	~	Water levels indicated on the soil boring logs are the levels measured in the	D TE	(PID)	Photo-Ionization Detector
SAMPI	Ring Sampler	Rock Core	WATE	borehole at the times indicated. Groundwater level variations will occur over time. In low permeability soils,	FIEL	(OVA)	Organic Vapor Analyzer
	S. S.			accurate determination of groundwater levels is not possible with short term water level observations.			
	Grab Sample	No Recovery					

DESCRIPTIVE SOIL CLASSIFICATION

Soil classification is based on the Unified Soil Classification System. Coarse Grained Soils have more than 50% of their dry weight retained on a #200 sieve; their principal descriptors are: boulders, cobbles, gravel or sand. Fine Grained Soils have less than 50% of their dry weight retained on a #200 sieve; they are principally described as clays if they are plastic, and silts if they are slightly plastic or non-plastic. Major constituents may be added as modifiers and minor constituents may be added according to the relative proportions based on grain size. In addition to gradation, coarse-grained soils are defined on the basis of their in-place relative density and fine-grained soils on the basis of their consistency.

LOCATION AND ELEVATION NOTES

Unless otherwise noted, Latitude and Longitude are approximately determined using a hand-held GPS device. The accuracy of such devices is variable. Surface elevation data annotated with +/- indicates that no actual topographical survey was conducted to confirm the surface elevation. Instead, the surface elevation was approximately determined from topographic maps of the area.

	(More than Density determine	NSITY OF COARSE-GRAI n 50% retained on No. 200 ed by Standard Penetration des gravels, sands and sil	sieve.) on Resistance	CONSISTENCY OF FINE-GRAINED SOILS (50% or more passing the No. 200 sieve.) Consistency determined by laboratory shear strength testing, field visual-manual procedures or standard penetration resistance						
TERMS	Descriptive Term (Density)	Standard Penetration or N-Value Blows/Ft.	Ring Sampler Blows/Ft.	Descriptive Term (Consistency)	Unconfined Compressive Strength, Qu, psf	Standard Penetration or N-Value Blows/Ft.	Ring Sampler Blows/Ft.			
뿔	Very Loose	0 - 3	0 - 6	Very Soft	less than 500	0 - 1	< 3			
	Loose	4 - 9	7 - 18	Soft	500 to 1,000	2 - 4	3 - 4			
TRENGT	Medium Dense	10 - 29	19 - 58	Medium-Stiff	1,000 to 2,000	4 - 8	5 - 9			
ြင	Dense	30 - 50	59 - 98	Stiff	2,000 to 4,000	8 - 15	10 - 18			
	Very Dense > 50		<u>≥</u> 99	Very Stiff	4,000 to 8,000	15 - 30	19 - 42			
				Hard	> 8,000	> 30	> 42			

RELATIVE PROPORTIONS OF SAND AND GRAVEL

<u>Descriptive Term(s)</u> of other constituents	Percent of Dry Weight	Major Component of Sample	Particle Size
Trace	< 15	Boulders	Over 12 in. (300 mm)
With	15 - 29	Cobbles	12 in. to 3 in. (300mm to 75mm)
Modifier	> 30	Gravel	3 in. to #4 sieve (75mm to 4.75 mm)
		Sand	#4 to #200 sieve (4.75mm to 0.075mm
		Silt or Clay	Passing #200 sieve (0.075mm)

GRAIN SIZE TERMINOLOGY

PLASTICITY DESCRIPTION

RELATIVE PROPORTIONS OF FINES

Descriptive Term(s) of other constituents	Percent of Dry Weight	Term	Plasticity Index
<u>or other constituents</u>	Diy Worgin	Non-plastic	0
Trace	< 5	Low	1 - 10
With	5 - 12	Medium	11 - 30
Modifier	> 12	High	> 30



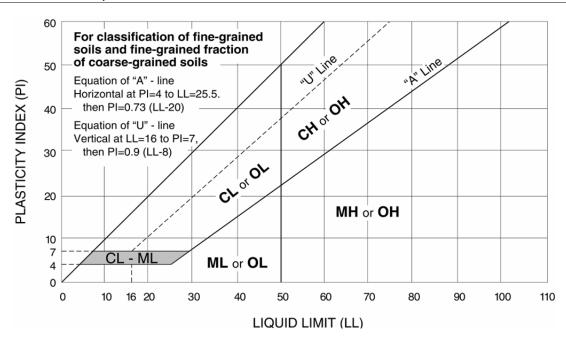
UNIFIED SOIL CLASSIFICATION SYSTEM

		Soil Classification			
Criteria for Assign	ning Group Symbols	and Group Names	s Using Laboratory Tests A	Group Symbol	Group Name ^B
	Gravels:	Clean Gravels:	Cu ≥ 4 and 1 ≤ Cc ≤ 3 ^E	GW	Well-graded gravel F
	More than 50% of	Less than 5% fines ^c	Cu < 4 and/or 1 > Cc > 3 ^E	GP	Poorly graded gravel F
	coarse fraction retained	Gravels with Fines:	Fines classify as ML or MH	GM	Silty gravel F,G,H
Coarse Grained Soils: More than 50% retained	on No. 4 sieve	More than 12% fines ^C	Fines classify as CL or CH	GC	Clayey gravel F,G,H
on No. 200 sieve	Sands:	Clean Sands:	Cu ≥ 6 and 1 ≤ Cc ≤ 3 ^E	SW	Well-graded sand I
011110. 200 01010	50% or more of coarse fraction passes No. 4	Less than 5% fines D	Cu < 6 and/or 1 > Cc > 3 ^E	SP	Poorly graded sand I
		Sands with Fines:	Fines classify as ML or MH	SM	Silty sand G,H,I
	sieve	More than 12% fines D	Fines classify as CL or CH	SC	Clayey sand G,H,I
	Silts and Clays: Liquid limit less than 50	Inorgania	PI > 7 and plots on or above "A" line J	CL	Lean clay K,L,M
		Inorganic:	PI < 4 or plots below "A" line J	ML	Silt K,L,M
		Onnonia	Liquid limit - oven dried	OL	Organic clay K,L,M,N
Fine-Grained Soils: 50% or more passes the		Organic:	Liquid limit - not dried < 0.75	OL	Organic silt K,L,M,O
No. 200 sieve		Inorganic:	PI plots on or above "A" line	CH	Fat clay K,L,M
	Silts and Clays:	illorganic.	PI plots below "A" line	MH	Elastic Silt K,L,M
	Liquid limit 50 or more	Organic:	Liquid limit - oven dried < 0.75	ОН	Organic clay K,L,M,P
		Organic.	Liquid limit - not dried < 0.75	ОП	Organic silt K,L,M,Q
Highly organic soils:	Primarily	organic matter, dark in o	color, and organic odor	PT	Peat

^A Based on the material passing the 3-inch (75-mm) sieve

^E
$$Cu = D_{60}/D_{10}$$
 $Cc = \frac{(D_{30})^2}{D_{10} \times D_{60}}$

Q PI plots below "A" line.





^B If field sample contained cobbles or boulders, or both, add "with cobbles or boulders, or both" to group name.

Gravels with 5 to 12% fines require dual symbols: GW-GM well-graded gravel with silt, GW-GC well-graded gravel with clay, GP-GM poorly graded gravel with silt, GP-GC poorly graded gravel with clay.
 Sands with 5 to 12% fines require dual symbols: SW-SM well-graded

^D Sands with 5 to 12% fines require dual symbols: SW-SM well-graded sand with silt, SW-SC well-graded sand with clay, SP-SM poorly graded sand with silt, SP-SC poorly graded sand with clay

 $^{^{\}text{F}}$ If soil contains \geq 15% sand, add "with sand" to group name.

^G If fines classify as CL-ML, use dual symbol GC-GM, or SC-SM.

^H If fines are organic, add "with organic fines" to group name.

¹ If soil contains ≥ 15% gravel, add "with gravel" to group name.

J If Atterberg limits plot in shaded area, soil is a CL-ML, silty clay.

K If soil contains 15 to 29% plus No. 200, add "with sand" or "with gravel," whichever is predominant.

^L If soil contains ≥ 30% plus No. 200 predominantly sand, add "sandy" to group name.

 $^{^{\}text{M}}$ If soil contains \geq 30% plus No. 200, predominantly gravel, add "gravelly" to group name.

 $^{^{}N}$ PI \geq 4 and plots on or above "A" line.

 $^{^{\}circ}$ PI < 4 or plots below "A" line.

P PI plots on or above "A" line.